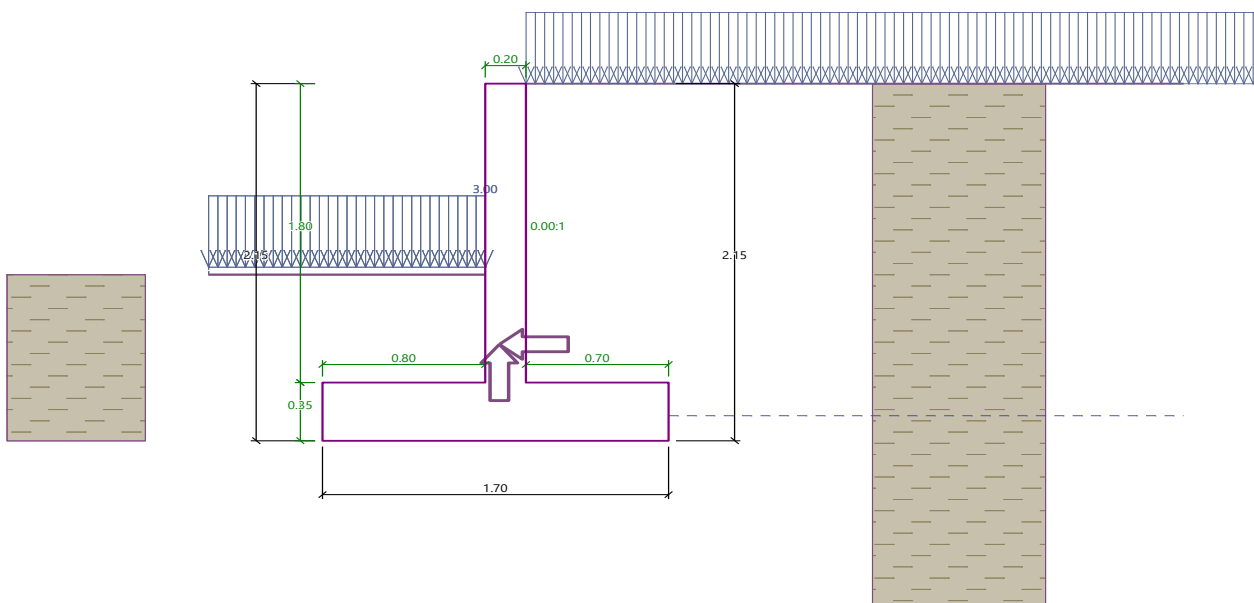


MEMORIAS DE CALCULO ESTRUCTURAL
PROYECTO
MURO DE CONTENCIÓN CANCHA MÚLTIPLE EL CARMELO

CARTAGENA, DEPARTAMENTO DE BOLÍVAR.



Responsable

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La Ceja - Antioquia

Octubre de 2023

Calculó: Ing. Jorge Ruiz A. – Ing. Roxana Quesada

Aprobó: Ing. Jorge Ruiz A.

Cantilever wall analysis

Input data

Project

Date : 17/10/2023

Settings

(input for current task)

Materials and standards

Concrete structures : ACI 318-19

Coefficients according to SNiP			
Combination coefficient :	$\gamma_k =$	1.00	[-]
Coefficient of action conditions :	$\gamma_c =$	0.90	[-]
Coefficient of importance of structure :	$\gamma_n =$	1.10	[-]

Wall analysis

Active earth pressure calculation : Coulomb
 Passive earth pressure calculation : Caquot-Kerisel
 Earthquake analysis : Mononobe-Okabe
 Shape of earth wedge : Calculate as skew
 Base key : The base key is considered as front face resistance
 Allowable eccentricity : 0.333
 Verification methodology : Safety factors (ASD)

Safety factors			
Permanent design situation			
Safety factor for overturning :	$SF_o =$	1.60	[-]
Safety factor for sliding resistance :	$SF_s =$	1.60	[-]
Safety factor for bearing capacity :	$SF_b =$	3.00	[-]

Material of structure

Unit weight $\gamma = 24.00 \text{ kN/m}^3$

Analysis of concrete structures carried out according to the standard ACI 318-11.

Concrete : $f'_c=28 \text{ MPa}$

Compressive strength $f'_c = 28.00 \text{ MPa}$

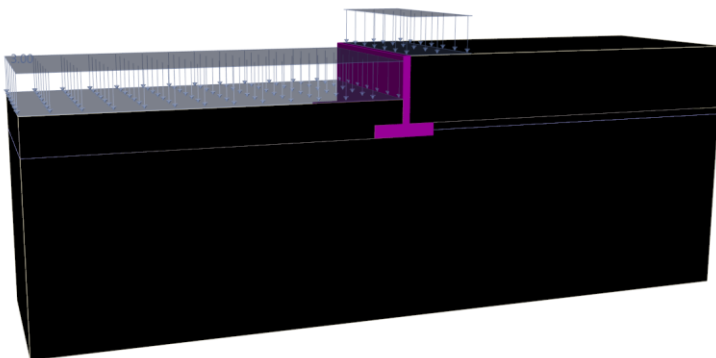
Longitudinal steel : $F_y=420 \text{ MPa}$ (user defined)

Tensile strength $f_y = 420.00 \text{ MPa}$

Geometry of structure

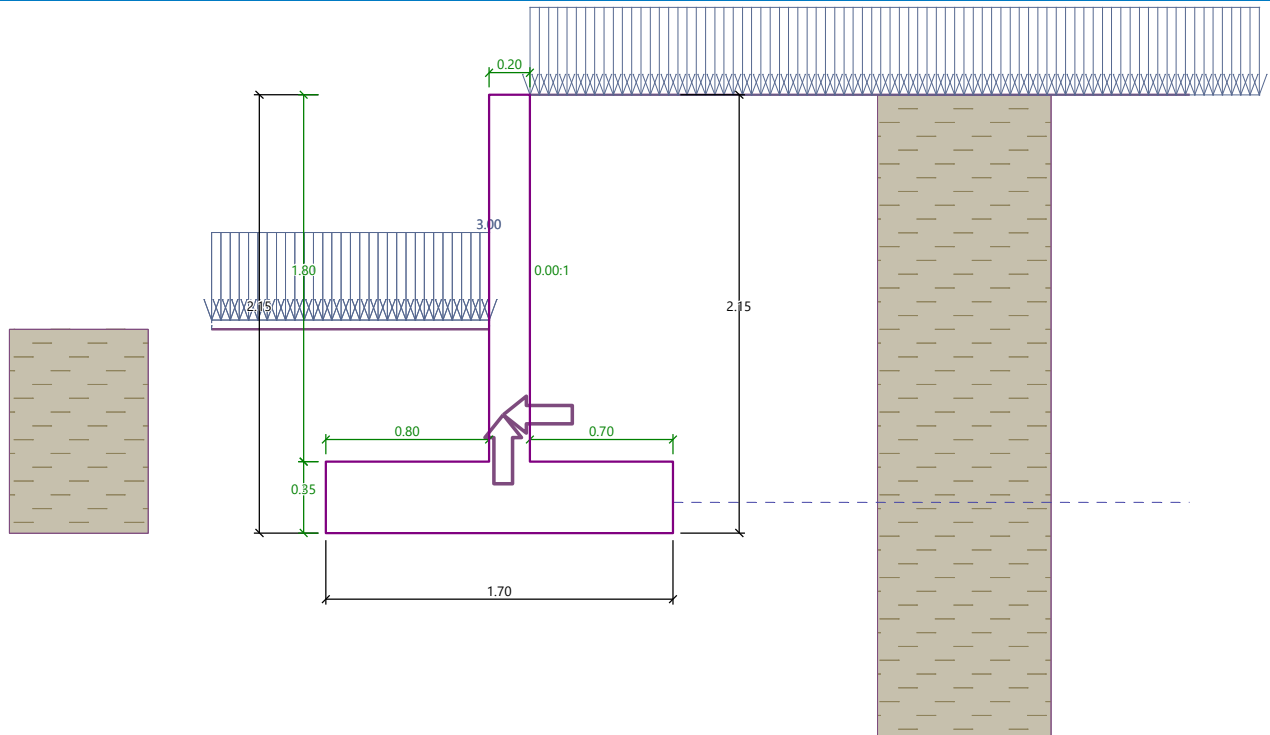
No.	Coordinate X [m]	Depth Z [m]
1	0.00	0.00
2	0.00	1.80
3	0.70	1.80
4	0.70	2.15
5	-1.00	2.15
6	-1.00	1.80
7	-0.20	1.80
8	-0.20	0.00

The origin [0,0] is located at the most upper right point of the wall.
 Wall section area = 0.95 m².

Name : Geometry	Stage - analysis : 1 - 0
Description : Wall Geometry	
	

Name : Geometry

Stage - analysis : 1 - 0



Basic soil parameters

No.	Name	Pattern	ϕ_{ef} [°]	c_{ef} [kPa]	γ [kN/m³]	γ_{su} [kN/m³]	δ [°]
1	CH		11.00	20.00	18.00	10.00	11.00

Soil parameters to compute pressure at rest

No.	Name	Pattern	Type calculation	ϕ_{ef} [°]	ν [-]	OCR [-]	K_r [-]
1	CH		cohesive	-	0.42	-	-

Soil parameters

CH

Unit weight : $\gamma = 18.00 \text{ kN/m}^3$

Stress-state : effective

Angle of internal friction : $\phi_{ef} = 11.00^\circ$

Cohesion of soil : $c_{ef} = 20.00 \text{ kPa}$

Angle of friction struc.-soil : $\delta = 11.00^\circ$

Soil : cohesive

Poisson's ratio : $\nu = 0.42$
 Saturated unit weight : $\gamma_{\text{sat}} = 20.00 \text{ kN/m}^3$

Geological profile and assigned soils

No.	Thickness of layer t [m]	Depth z [m]	Assigned soil	Pattern
1	-	0.00 .. ∞	CH	

Foundation

Type of foundation : soil from geological profile

Terrain profile

Terrain behind the structure is flat.

Water influence

GWT behind the structure lies at a depth of 2.00 m
 Uplift in foot. bottom due to different pressures is not considered.
 The evolution of tensile cracks is considered in the analyses. Depth of cracks is 2.00 m.

Input surface surcharges

No.	Surcharge		Action	Mag.1 [kN/m ²]	Mag.2 [kN/m ²]	Ord.x x [m]	Length l [m]	Depth z [m]
	new	change						
1	Yes		permanent	3.00				on terrain

Resistance on front face of the structure

Resistance on front face of the structure: passive
 Soil on front face of the structure - CH
 Angle of friction struc.-soil $\delta = 0.00^\circ$
 Soil thickness in front of structure $h = 1.00 \text{ m}$
 Terrain surcharge $f = 3.00 \text{ kN/m}^2$

Terrain in front of structure is flat.

Earthquake

Factor of horizontal acceleration $K_h = 0.1000$
 Factor of vertical acceleration $K_v = 0.0500$

Water below the GWT is restricted.

Settings of the stage of construction

Design situation : permanent
 The wall is free to move. Active earth pressure is therefore assumed.

Verification No. 1

Passive pressure on front face of the structure - partial results

Layer No.	Thickness [m]	α [°]	φ_d [°]	c_d [kPa]	γ [kN/m³]	δ_d [°]	K_p	Comment
1	0.65	0.00	11.00	20.00	18.00	0.00	1.481	
2	0.00	89.90(11.00)	11.00	20.00	18.00	0.00	1.344	MODIFIED
3	0.35	0.00	11.00	20.00	18.00	0.00	1.481	

Passive pressure distribution on front face of the structure

Layer No.	Start [m] End [m]	σ_z [kPa]	σ_w [kPa]	Pressure [kPa]	Hor. comp. [kPa]	Vert. comp. [kPa]
1	0.00	0.00	0.00	48.68	48.68	0.00
	0.65	11.70	0.00	66.00	66.00	0.00
2	0.65	11.70	0.00	62.09	0.10	62.09
	0.65	11.72	0.00	62.12	0.10	62.12
3	0.65	11.72	0.00	66.04	66.04	0.00
	1.00	18.00	0.00	75.33	75.33	0.00

Active pressure behind the structure - partial results

Layer No.	Thickness [m]	α [°]	φ_d [°]	c_d [kPa]	γ [kN/m³]	δ_d [°]	K_a	Comment
1	0.95	0.00	11.00	20.00	18.00	11.00	0.609	
2	0.05	39.50	11.00	20.00	18.00	11.00	1.068	
3	0.09	39.50	11.00	20.00	18.00	11.00	1.068	
4	0.71	39.50	11.00	20.00	18.00	11.00	1.068	
5	0.20	0.00	11.00	20.00	18.00	11.00	0.609	
6	0.15	0.00	11.00	20.00	10.00	11.00	0.609	

Active pressure distribution behind the structure (without surcharge)

Layer No.	Start [m] End [m]	σ_z [kPa]	σ_w [kPa]	Pressure [kPa]	Hor. comp. [kPa]	Vert. comp. [kPa]
1	0.00	0.00	0.00	0.00	0.00	0.00
	0.95	17.11	0.00	0.00	0.00	0.00
2	0.95	17.11	0.00	0.00	0.00	0.00
	1.00	18.00	0.00	0.00	0.00	0.00
3	1.00	18.00	0.00	0.00	0.00	0.00
	1.09	19.56	0.00	0.00	0.00	0.00

Layer No.	Start [m] End [m]	σ_z [kPa]	σ_w [kPa]	Pressure [kPa]	Hor. comp. [kPa]	Vert. comp. [kPa]
4	1.09	19.56	0.00	0.00	0.00	0.00
	1.80	32.40	0.00	13.71	8.72	10.58
5	1.80	32.40	0.00	0.00	0.00	0.00
	2.00	36.00	0.00	0.00	0.00	0.00
6	2.00	36.00	0.00	0.00	0.00	0.00
	2.15	37.50	1.50	0.00	0.00	0.00

Forces acting on construction

Name	F_{hor} [kN/m]	App.Pt. z [m]	F_{vert} [kN/m]	App.Pt. x [m]	Design coefficient
Weight - wall	0.00	-0.58	22.92	0.87	1.000
Earthq.- constr.	2.29	-0.58	-1.15	0.87	1.000
FF resistance	-61.91	-0.46	0.08	0.40	1.000
Resistance on front face	-4.44	-0.50	0.01	0.40	1.000
Earthq.- face	1.27	-0.67	0.00	0.80	1.000
Weight - earth wedge	0.00	-0.63	5.35	1.23	1.000
Earthquake - soil wedge	0.54	-0.63	-0.27	1.23	1.000
Active pressure	3.11	-0.59	3.77	1.50	1.000
Water pressure	0.11	-0.05	0.00	1.00	1.000
Tensile crack	20.00	-0.82	0.00	1.00	1.000
Uplift pressure	0.00	-2.15	0.00	1.00	1.000
Earthq.- act.pressure	4.94	-1.45	2.34	1.20	1.000
Surch.1 - surface	1.62	-0.75	2.55	1.32	1.000

Verification of complete wall

Check for overturning stability

Resisting moment $M_{res} = 30.34$ kNm/m

Overturning moment $M_{ovr} = -1.93$ kNm/m

Safety factor = 1000.00 > 1.60

Wall for overturning is SATISFACTORY

Check for slip

Resisting horizontal force $H_{res} = 33.48$ kN/m

Active horizontal force $H_{act} = -32.47$ kN/m

Safety factor = 1000.00 > 1.60

Wall for slip is SATISFACTORY

Overall check - WALL is SATISFACTORY

Warning - allowable range of input data exceeded during earthquake analysis!

The analysis is carried out with the modified value of terrain inclination β .

Bearing capacity of foundation soil

Design load acting at the center of footing bottom

No.	Moment [kNm/m]	Norm. force [kN/m]	Shear Force [kN/m]	Eccentricity [-]	Stress [kPa]
1	-8.75	35.61	-32.47	0.000	20.95

Service load acting at the center of footing bottom

No.	Moment [kNm/m]	Norm. force [kN/m]	Shear Force [kN/m]
1	-8.75	35.61	-32.47

Verification of foundation soil

Stress in the footing bottom : rectangle

Eccentricity verification

Max. eccentricity of normal force $e = 0.000$

Maximum allowable eccentricity $e_{alw} = 0.333$

Eccentricity of the normal force is SATISFACTORY

Verification of bearing capacity

Max. stress at footing bottom $\sigma = 20.95 \text{ kPa}$

Bearing capacity of foundation soil $R_d = 277.50 \text{ kPa}$

Safety factor = 13.25 > 3.00

Bearing capacity of foundation soil is SATISFACTORY

Overall verification - bearing capacity of found. soil is SATISFACTORY

Dimensioning No. 1

Wall stem check - front reinf.

Passive pressure on front face of the structure - partial results

Layer No.	Thickness [m]	α [°]	φ_d [°]	c_d [kPa]	γ [kN/m³]	δ_d [°]	K_p	Comment
1	0.65	0.00	11.00	20.00	18.00	0.00	1.481	

Passive pressure distribution on front face of the structure

Layer No.	Start [m] End [m]	σ_z [kPa]	σ_w [kPa]	Pressure [kPa]	Hor. comp. [kPa]	Vert. comp. [kPa]
1	0.00	0.00	0.00	48.68	48.68	0.00
	0.65	11.68	0.00	65.97	65.97	0.00

Pressure at rest behind the structure - partial results

Layer No.	Thickness [m]	α [°]	φ_d [°]	c_d [kPa]	γ [kN/m ³]	K_r	Comment
1	1.00	0.00	11.00	20.00	18.00	0.724	
2	0.80	0.00	11.00	20.00	18.00	0.724	

Pressure at rest distribution behind the structure (without surcharge)

Layer No.	Start [m] End [m]	σ_z [kPa]	σ_w [kPa]	Pressure [kPa]	Hor. comp. [kPa]	Vert. comp. [kPa]
1	0.00	0.00	0.00	0.00	0.00	0.00
	1.00	18.00	0.00	13.03	13.03	0.00
2	1.00	18.00	0.00	13.03	13.03	0.00
	1.80	32.38	0.00	23.45	23.45	0.00

Forces acting on construction

Name	F_{hor} [kN/m]	App.Pt. z [m]	F_{vert} [kN/m]	App.Pt. x [m]	Design coefficient
Weight - wall	0.00	-0.90	8.63	0.10	1.000
Earthq.- constr.	0.86	-0.90	-0.43	0.10	1.000
FF resistance	-37.19	-0.31	0.00	0.00	1.000
Resistance on front face	-2.88	-0.32	0.00	0.00	1.000
Earthq.- face	0.53	-0.43	0.00	0.00	1.000
Pressure at rest	21.09	-0.60	0.00	0.20	1.000
Water pressure	0.00	-1.80	0.00	0.20	1.000
Tensile crack	16.18	-0.60	0.00	0.20	1.000
Earthquake - pressure at rest	5.53	-0.90	0.00	0.20	1.000
Surch.1 - surface	3.91	-0.90	0.00	0.20	1.000

Wall stem check - front reinf.

Front reinforcement is not required.

Wall stem check - back reinf.

Passive pressure on front face of the structure - partial results

Layer No.	Thickness [m]	α [°]	φ_d [°]	c_d [kPa]	γ [kN/m ³]	δ_d [°]	K_p	Comment
1	0.65	0.00	11.00	20.00	18.00	0.00	1.481	

Passive pressure distribution on front face of the structure

Layer No.	Start [m] End [m]	σ_z [kPa]	σ_w [kPa]	Pressure [kPa]	Hor. comp. [kPa]	Vert. comp. [kPa]
1	0.00	0.00	0.00	48.68	48.68	0.00
	0.65	11.68	0.00	65.97	65.97	0.00

Pressure at rest behind the structure - partial results

Layer No.	Thickness [m]	α [°]	φ_d [°]	c_d [kPa]	γ [kN/m ³]	K_r	Comment
1	1.00	0.00	11.00	20.00	18.00	0.724	
2	0.80	0.00	11.00	20.00	18.00	0.724	

Pressure at rest distribution behind the structure (without surcharge)

Layer No.	Start [m] End [m]	σ_z [kPa]	σ_w [kPa]	Pressure [kPa]	Hor. comp. [kPa]	Vert. comp. [kPa]
1	0.00	0.00	0.00	0.00	0.00	0.00
	1.00	18.00	0.00	13.03	13.03	0.00
2	1.00	18.00	0.00	13.03	13.03	0.00
	1.80	32.38	0.00	23.45	23.45	0.00

Forces acting on construction

Name	F_{hor} [kN/m]	App.Pt. z [m]	F_{vert} [kN/m]	App.Pt. x [m]	Design coefficient
Weight - wall	0.00	-0.90	8.63	0.10	1.000
Earthq.- constr.	0.86	-0.90	-0.43	0.10	1.000
FF resistance	-37.19	-0.31	0.00	0.00	1.000
Resistance on front face	-2.88	-0.32	0.00	0.00	1.000
Earthq.- face	0.53	-0.43	0.00	0.00	1.000
Pressure at rest	21.09	-0.60	0.00	0.20	1.000
Water pressure	0.00	-1.80	0.00	0.20	1.000
Tensile crack	16.18	-0.60	0.00	0.20	1.000
Earthquake - pressure at rest	5.53	-0.90	0.00	0.20	1.000
Surch.1 - surface	3.91	-0.90	0.00	0.20	1.000

Wall stem check - back reinf.

Wall check at the construction joint 1.80 m from the wall crest

Reinforcement and dimensions of the cross-section

4 prof. No. 4, cover 75.0 mm

Inputted reinforcement area = 506.7 mm²

Required reinforcement area = 448.6 mm²

Cross-section width = 1.00 m

Cross-section height = 0.20 m

Reinforcement ratio $\rho = 0.43 \% > 0.33 \% = \rho_{min}$
 Position of neutral axis $c = 0.02 \text{ m} < 0.05 \text{ m} = c_{max}$
 Ultimate shear force $\phi V_n = 78.20 \text{ kN} > 8.03 \text{ kN} = V_u$
 Ultimate moment $\phi M_n = 34.88 \text{ kNm} > 19.45 \text{ kNm} = M_u$

Cross-section is SATISFACTORY.

Wall stem check - back reinf. - V_u

Wall check at the construction joint 1.16 m from the wall crest

Reinforcement and dimensions of the cross-section

4 prof. No. 4, cover 75.0 mm

Inputted reinforcement area = 506.7 mm²

Required reinforcement area = 448.6 mm²

Cross-section width = 1.00 m

Cross-section height = 0.20 m

Ultimate shear force $\phi V_n = 78.20 \text{ kN} > 20.25 \text{ kN} = V_u$

Cross-section is SATISFACTORY.

Wall jump check

Forces acting on construction

Name	F_{hor} [kN/m]	App.Pt. z [m]	F_{vert} [kN/m]	App.Pt. x [m]	Design coefficient
Weight - wall	0.00	-0.58	22.92	0.87	1.000
Earthq.- constr.	2.29	-0.58	-1.15	0.87	1.000
FF resistance	-61.91	-0.46	0.08	0.40	1.000
Resistance on front face	-4.44	-0.50	0.01	0.40	1.000
Earthq.- face	1.27	-0.67	0.00	0.80	1.000
Weight - earth wedge	0.00	-0.63	5.35	1.23	1.000
Earthquake - soil wedge	0.54	-0.63	-0.27	1.23	1.000
Active pressure	3.11	-0.59	3.77	1.50	1.000
Water pressure	0.11	-0.05	0.00	1.00	1.000
Tensile crack	20.00	-0.82	0.00	1.00	1.000
Uplift pressure	0.00	-2.15	0.00	1.00	1.000
Earthq.- act.pressure	4.94	-1.45	2.34	1.20	1.000
Surch.1 - surface	1.62	-0.75	2.55	1.32	1.000

Wall jump check

Reinforcement and dimensions of the cross-section

4 prof. No. 4, cover 75.0 mm

Inputted reinforcement area = 506.7 mm²

Required reinforcement area = 52.6 mm²
 Cross-section width = 1.00 m
 Cross-section height = 0.35 m

Reinforcement ratio ρ = 0.19 % > 0.02 % = ρ_{min}
 Position of neutral axis c = 0.01 m < 0.12 m = c_{max}
 Ultimate shear force ϕV_n = 177.06 kN > 10.04 kN = V_u
 Ultimate moment ϕM_n = 50.60 kNm > 4.01 kNm = M_u

Cross-section is SATISFACTORY.

Wall heel check

Passive pressure on front face of the structure - partial results

Layer No.	Thickness [m]	α [°]	Φ_d [°]	c_d [kPa]	γ [kN/m ³]	δ_d [°]	K_p	Comment
1	0.65	0.00	11.00	20.00	18.00	0.00	1.481	
2	0.00	89.90(11.00)	11.00	20.00	18.00	0.00	1.344	MODIFIED
3	0.35	0.00	11.00	20.00	18.00	0.00	1.481	

Passive pressure distribution on front face of the structure

Layer No.	Start [m] End [m]	σ_z [kPa]	σ_w [kPa]	Pressure [kPa]	Hor. comp. [kPa]	Vert. comp. [kPa]
1	0.00	0.00	0.00	48.68	48.68	0.00
	0.65	11.70	0.00	66.00	66.00	0.00
2	0.65	11.70	0.00	62.09	0.10	62.09
	0.65	11.72	0.00	62.12	0.10	62.12
3	0.65	11.72	0.00	66.04	66.04	0.00
	1.00	18.00	0.00	75.33	75.33	0.00

Active pressure behind the structure - partial results

Layer No.	Thickness [m]	α [°]	Φ_d [°]	c_d [kPa]	γ [kN/m ³]	δ_d [°]	K_a	Comment
1	0.95	0.00	11.00	20.00	18.00	11.00	0.609	
2	0.05	39.50	11.00	20.00	18.00	11.00	1.068	
3	0.09	39.50	11.00	20.00	18.00	11.00	1.068	
4	0.71	39.50	11.00	20.00	18.00	11.00	1.068	
5	0.20	0.00	11.00	20.00	18.00	11.00	0.609	
6	0.15	0.00	11.00	20.00	10.00	11.00	0.609	

Active pressure distribution behind the structure (without surcharge)

Layer No.	Start [m] End [m]	σ_z [kPa]	σ_w [kPa]	Pressure [kPa]	Hor. comp. [kPa]	Vert. comp. [kPa]
1	0.00	0.00	0.00	0.00	0.00	0.00
	0.95	17.11	0.00	0.00	0.00	0.00
2	0.95	17.11	0.00	0.00	0.00	0.00
	1.00	18.00	0.00	0.00	0.00	0.00
3	1.00	18.00	0.00	0.00	0.00	0.00
	1.09	19.56	0.00	0.00	0.00	0.00
4	1.09	19.56	0.00	0.00	0.00	0.00
	1.80	32.40	0.00	13.71	8.72	10.58
5	1.80	32.40	0.00	0.00	0.00	0.00
	2.00	36.00	0.00	0.00	0.00	0.00
6	2.00	36.00	0.00	0.00	0.00	0.00
	2.15	37.50	1.50	0.00	0.00	0.00

Forces acting on construction

Name	F_{hor} [kN/m]	App.Pt. z [m]	F_{vert} [kN/m]	App.Pt. x [m]	Design coefficient
Weight - wall	0.00	-0.17	5.88	1.35	1.000
Weight - earth wedge	0.00	-0.63	5.35	1.23	1.000
Active pressure	3.11	-0.59	3.77	1.50	1.000
Tensile crack	20.00	-0.82	0.00	1.00	1.000
Surch.1 - surface	1.62	-0.75	2.55	1.32	1.000
Contact stress	0.00	0.00	-22.14	1.38	1.000

Wall heel check

Reinforcement and dimensions of the cross-section

4 prof. No. 4, cover 75.0 mm

Inputted reinforcement area = 506.7 mm²

Required reinforcement area = 0.0 mm²

Cross-section width = 1.00 m

Cross-section height = 0.35 m

Fibers loaded in tension are on the front side of the cross-section, the cross-section cannot be checked using this program.

Name : Dimensioning

Stage - analysis : 1 - 1

